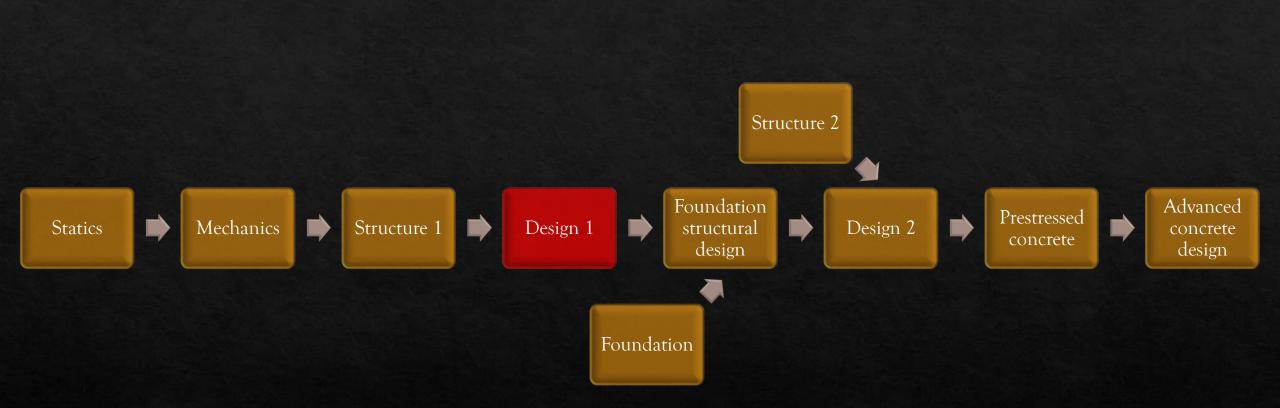


#### Reinforced Concrete Design I ENCE 335 Introduction

Dr. Khalil M. Qatu



You are here !!

## What is reinforced concrete (RC)?

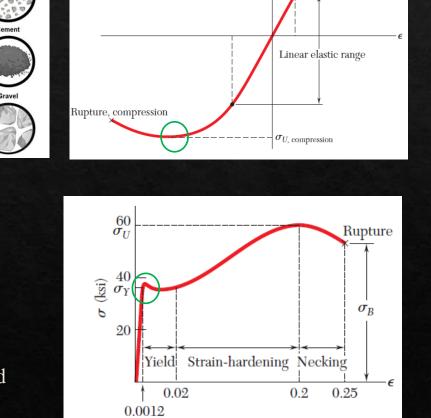
- ♦ Concrete is very durable and easy to construct material
- Relatively cheap
- ♦ High \_\_\_\_\_ Strength
- ♦ Strength range
  - $\diamond$  Normal Strength 10 40 MPa
  - ♦ High strength 40-150 MPa
- ♦ BUT ??

- ♦ Steel is more expensive
- ♦ High \_\_\_\_\_ strength



Concrete

- Reinforced concrete takes the durability and affordability of concrete and high strength and ductility of steel
- ♦ Location of needed steel reinforcement ??



 $\sigma_{U \text{ tension}}$ 

3

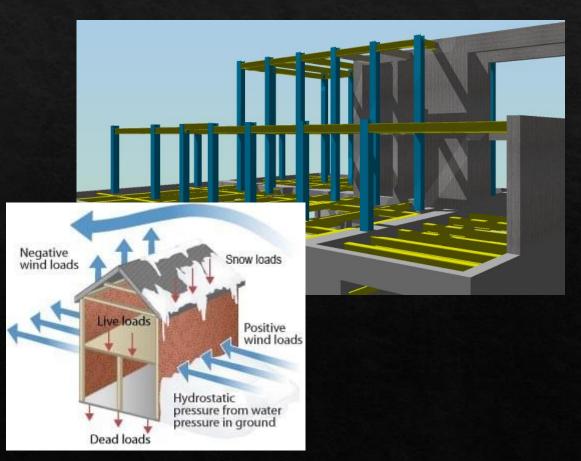
Rupture, tension

#### What is reinforced concrete?

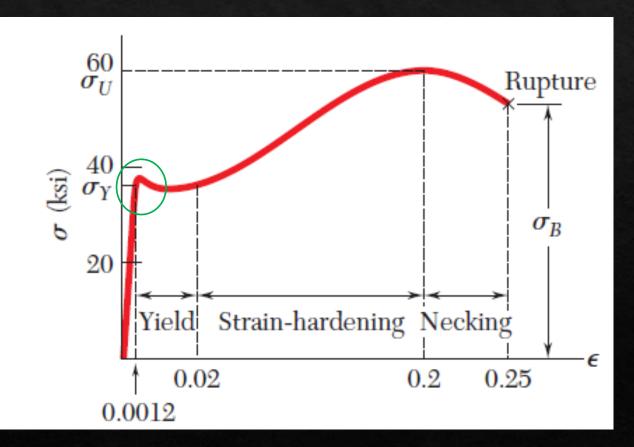
Product	ASTM Specification	Designation	Minimum Yield Strength, MPa (psi)	Minimum Tensile Strength, MPa (psi)
Reinforcing bars	A615	Grade 280 Grade 420 Grade 520	280 (40,000) 420 (60,000) 520 (75,000)	420 (60,000) 620 (90,000) 690 (100,000)
	A706	Grade 420	420 (60,000) [540 (78,000) maximum]	550 (80,000) <sup>a</sup>
	A996	Grade 280 Grade 350 Grade 420	280 (40,000) 350 (50,000) 420 (60,000)	420 (60,000) 550 (80,000) 620 (90,000)
	A1035	Grade 690	690 (100,000)	1030 (150,000)

# What is Design ??

- ♦ Design is the process of devising a system and/or a component to meet desired demand
- ♦ In Civil Engineering :
  - ♦ The system:
  - ♦ The demand:
- Reinforced Concrete Design ??
  - Determine Geometry of concrete member
    Amount and location of steel reinforcement

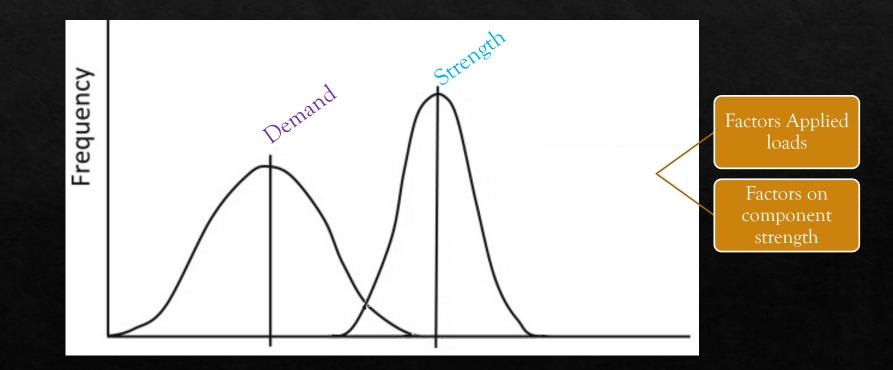


Allowable stress design (ASD)



♦ Load and resistance factored design (LRFD)

♦ This methods considers the variability in the applied loads (internal) and component strength



♦ Load and resistance factored design (LRFD)

♦ Load Factors and load combinations

Table 5.3.1—Load combinations				
Load combination	Equation	Primary load		
U=1.4D	(5.3.1a)	D		
$U = 1.2D + 1.6L + 0.5(L_r \text{ or } S \text{ or } R)$	(5.3.1b)	L		
$U = 1.2D + 1.6(L_r \text{ or } S \text{ or } R) + (1.0L \text{ or } 0.5W)$	(5.3.1c)	$L_r$ or $S$ or $R$		
$U = 1.2D + 1.0W + 1.0L + 0.5(L_r \text{ or } S \text{ or } R)$	(5.3.1d)	W		
U = 1.2D + 1.0E + 1.0L + 0.2S	(5.3.1e)	Ε		
U = 0.9D + 1.0W	(5.3.1f)	W		
U = 0.9D + 1.0E	(5.3.1g)	Ε		

Note that factors are higher for loads with high variability

♦ After structural analysis we get ultimate internal forces in the member in consideration

$$M_u V_u P_u$$

#### Minimum uniformly distributed live loads

	ve Load, kN/m²	Occupancy or Use	Live Load, kN/m²
Apartments (see residential)		Dining rooms and restaurants	4.8
Access floor systems		Dwellings (see residential)	
Office use	2.4	Fire escapes	4.8
Computer use	4.8	On single-family dwellings only	1.9
Armories and drill rooms	7.2	Garages (passenger cars only)	1.9
Assembly areas and theaters		Trucks and buses <sup>a</sup>	
Fixed seats (fastened to floor)	2.9	Grandstands (see stadium and arena bleachers)	
Lobbies	4.8	Gymnasiums, main floors and balconies <sup>b</sup>	4.8
Movable seats	4.8	Hospitals	
Platforms (assembly)	4.8	Operating rooms, laboratories	2.9
Stage floors	7.2	Patient rooms	1.9
Balconies (exterior)	4.8	Corridors above first floor	3.8
On one and two-family residences	2.9	Hotels (see residential)	
only, and not exceeding 9.3 m <sup>2</sup>		Libraries	
Bowling alleys, poolrooms, and similar	3.6	Reading rooms	2.9
recreational areas		Stack rooms <sup>c</sup>	7.2
Catwalks for maintenance access	1.9	Corridors above first floor	3.8
Corridors	117	Manufacturing	010
First floor	4.8	Light	6.0
Other floors, same as occupancy	4.0	Heavy	12.0
served except as indicated		Marquees and canopies	3.6
Dance halls and ballrooms	4.8	Office buildings	5.0
Decks (patio and roof)	4.0	File and computer rooms shall be designed for	
Same as area served, or for the		heavier loads based on anticipated occupancy	
type of occupancy accommodated		Lobbies and first-floor corridors	4.8
11 1 1	2.4	Schools	
Offices		Classrooms	1.9
Corridors above first floor	3.8	010000000	
Penal institutions		Corridors above first floor	3.8
Cell blocks	1.9	First-floor corridors	4.8
Corridors	4.8	Sidewalks, vehicular driveways, and yards	12.0
Residential		subject to trucking <sup>d</sup>	
Dwellings (one and two-family)		Stadiums and arenas	
Uninhabitable attics without storage	0.5	Bleachers <sup>b</sup>	4.8
Uninhabitable attics with storage	1.0	Fixed seats (fastened to floor) <sup>b</sup>	2.9
Habitable attics and sleeping areas	1.4	Stairs and exit ways	4.8
All other areas except stairs and balconies	1.9	One and two-family residences only	1.9
Hotels and multifamily houses		Storage areas above ceilings	1.0
Private rooms and corridors serving them	1.9	Storage warehouses (shall be designed for	
Public rooms and corridors serving them	4.8	heavier loads if required for anticipated storage	
Reviewing stands, grandstands, and bleachers <sup>b</sup>		Light	6.0
Roofs		Heavy	12.0
Ordinary flat, pitched, and curved roofs	1.0	Stores	
Roofs used for promenade purposes	2.9	Retail	
Roofs used for roof gardens or assembly purpose	4.8	First floor	4.8
Roofs used for other special purposes"		Upper floors	3.6
Awnings and canopies		Wholesale, all floors	6.0
Fabric construction supported by a	0.25	Walkways and elevated platforms	2.9
		(at a share and the second	
lightweight rigid skeleton structure <sup>f</sup> All other construction		(other than exitways)	4.8

♦ Load and resistance factored design (LRFD)

 $\diamond$  Strength reduction factors ( $\phi$ )

 $\phi M_n \qquad \phi V_n \qquad \phi P_n$ 

 We get Nominal strength is calculated using concepts from Statics & Mechanics of Materials

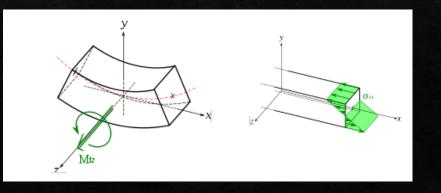


Table 21.2.1—Strength reduction factors $\phi$							
Action or structural element		φ	Exceptions				
(a)	Moment, axial force, or combined moment and axial force	0.65 to 0.90 in accordance with 21.2.2	Near ends of preten- sioned members where strands are not fully developed, $\phi$ shall be in accordance with 21.2.3.				
(b)	Shear	0.75	Additional requirements are given in 21.2.4 for structures designed to resist earthquake effects.				
(c)	Torsion	0.75					
(d)	Bearing	0.65	—				
(e)	Post-tensioned anchorage zones	0.85	_				
(f)	Brackets and corbels	0.75	—				
(g)	Struts, ties, nodal zones, and bearing areas designed in accordance with strut-and- tie method in Chapter 23	0.75	_				
(h)	Components of connec- tions of precast members controlled by yielding of steel elements in tension	0.90	_				
(i)	Plain concrete elements	0.60	_				
(j)	Anchors in concrete elements	0.45 to 0.75 in accor- dance with Chapter 17	_				

# Design CODE

♦ In our region we use the ACI code 318-14 (or 19??)

- ♦ Load Factors
- Strength requirements
- Serviceability requirements
- Reinforcement limits
- Reinforcement detailing

CHAPTER 3—REFERENCED STANDARDS CHAPTER 4—STRUCTURAL SYSTEM REQUIREMENTS CHAPTER 5—LOADS CHAPTER 6—STRUCTURAL ANALYSIS CHAPTER 7—ONE-WAY SLABS CHAPTER 8—TWO-WAY SLABS CHAPTER 9—BEAMS CHAPTER 10—COLUMNS CHAPTER 11—WALLS CHAPTER 12—DIAPHRAGMS CHAPTER 13—FOUNDATIONS CHAPTER 14—PLAIN CONCRETE CHAPTER 15—BEAM-COLUMN AND SLAB-COLUMN JOINTS CHAPTER 16—CONNECTIONS BETWEEN MEMBERS CHAPTER 17—ANCHORING TO CONCRETE CHAPTER 18—EARTHQUAKE-RESISTANT STRUCTURES CHAPTER 19—CONCRETE: DESIGN AND DURA CHAPTER 20—STEEL REINFORCEMENT PROPERTIES, DURABILITY, AND CHAPTER 21—STRENGTH REDUCTION FACTORS CHAPTER 22—SECTIONAL STRENGTH CHAPTER 23—STRUT-AND-TIE MODELS CHAPTER 24—SERVICEABILITY REQUIREMENTS CHAPTER 25—REINFORCEMENT DETAILS CHAPTER 26—CONSTRUCTION DOCU CHAPTER 27—STRENGTH EVALUATION OF EXISTING STRUCTURES

#### An ACI Standard and Report

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Building Code Requirements for Structural Concrete (ACI 318-14)

Commentary on Building Code Requirements for Structural Concrete (ACI 318R-14)

Reported by ACI Committee 318

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American Concrete Institute